

## STRUCTURAL BEHAVIOUR OF FULLY COUPLED SPAR–MOORING SYSTEM UNDER EXTREME WAVE LOADING

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**Abstract.** Floating spar platform has been proven to be an economical and efficient type of offshore oil and gas exploration structure in deep and ultra-deep seas. Associated nonlinearities, coupled action, damping effect and extreme sea environments may modify its structural responses. In this study, fully coupled spar–mooring system is modelled integrating mooring lines with the cylindrical spar hull. Rigid beam element simulates large cylindrical spar hull and catenary mooring lines are configured by hybrid beam elements. Nonlinear finite element analysis is performed under extreme wave loading at severe deep sea. Morison's equation has been used to calculate the wave forces. Spar responses and mooring line tensions have been evaluated. Though the maximum mooring line tensions are larger at severe sea-state, it becomes regular after one hour of wave loading. The response time histories in surge, heave, pitch and the maximum mooring tension gradually decreases even after attaining steady state. It is because of damping due to heavier and longer mooring lines in coupled spar–mooring system under deep water conditions. The relatively lesser values of response time histories in surge, heave, pitch and the maximum mooring tension under extreme wave loading shows the suitability of a spar platform for deep water harsh and uncertain environmental conditions.

**Keywords:** floating structures, spar platform, fully coupled, nonlinear dynamic response, mooring tension, extreme wave loading.

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### Introduction

Offshore oil and gas exploration from shallow and intermediate water depths is traditionally carried out using the conventional jacket type fixed platforms. As the water depth increases, fixed platforms becomes uneconomical and the prominence shifts to floating production systems (Hillis, Courtney 2011; Islam *et al.* 2012a, b). A spar platform is a compliant floating structure used for deep water applications of drilling, production, processing and storage plus off-loading of ocean deposits (Halkyard 1996; Islam *et al.* 2011a). Numerous studies have recently been performed in order to assess the effect of coupling on different offshore floating production systems/a spar buoy (Chen *et al.* 2001; Culla, Carcaterra 2007; Ran *et al.* 1996). Ma and Patel (2001) have conducted parametric studies on Spar and TLP for different depths. Sarkar and Roesset (2004), Grigorenko and Yaremchenko (2009), Kim and Lee (2011), Noorzaei *et al.* (2010) carried out static as well as dynamic analysis for differ-

ent environmental conditions and evaluated the response behaviour. Low and Langley (2007) have compared the methods for the couple analysis of floating structures. Coupled dynamic behaviours of hull/mooring/riser of a spar platform have correspondingly been investigated by several researchers (Chen *et al.* 2006; Kim *et al.* 2005, 2001; Islam *et al.* 2011b). Chen *et al.* (1999) presented the response of a spar constrained by slack mooring lines to steep ocean waves by two different schemes: a quasi-static approach (SMACOS), and a coupled dynamic approach (COUPLE) to reveal the coupling effects between spar and its mooring system. In coupled dynamic approach, dynamics of a mooring system is calculated using a numerical program, known as CABLE3D.

Ding *et al.* (2003) presented a numerical code (COUPLE6D) for the coupled dynamic analysis of moored offshore structures. Tahar and Kim (2008) developed a numerical tool for the coupled analysis of a deep water floating platform with polyester mooring lines. Low and

Langley (2008) presented a hybrid time/frequency domain approach for the coupled analysis of vessel/mooring/riser. The vessel was modelled as a rigid body with six degrees-of-freedom, and the lines were discretized as lumped masses connected by linear extensional and rotational springs. The method was found to be in good agreement with the fully coupled time domain analysis for relatively shallow water depths. Montasir and Kurian (2011) evaluated the effect of slowly varying drift forces on the motion characteristics of truss spar platforms. Yang and Kim (2010) carried out the coupled analysis of a hull-tendon-riser for a TLP. The mooring line/riser/tendon system was modelled as an elastic rod. It was connected to the hull by linear and rotational springs. The equilibrium equations of hull and mooring line/risers/tendon system were solved simultaneously. Jameel et al. (2012, 2013) evaluated coupled spar responses considering essential nonlinearities.

Though application of spar platforms is rapidly increasing all over the world, there is a lack of precise modelling and nonlinear coupled response investigation in extreme sea environments. Furthermore, contribution of moorings in terms of drag, inertia and damping for their longer lengths, larger sizes and heavier weights are not fully incorporated, which is more pronounced in deep water conditions. Hence, the main objective of this study is to idealize the spar mooring integrated system as a fully coupled structure; to study the damping effects of mooring lines and to investigate the nonlinear responses under extreme wave loading. Nonlinear coupled responses under the severe sea state have been evaluated in the form of translational motion at surge, heave and rotational motion in pitch direction along with the mooring line tension.

## 1. Mathematical model

The non-linear deterministic model (Fig. 1) for the coupled dynamic analysis includes the formulation of a non-linear stiffness matrix allowing for mooring line tension fluctuations subjected to variable buoyancy as well as structural and environmental nonlinearities. The model involves selection and solution of wave theory that reasonably represents the water particle kinematics to es-

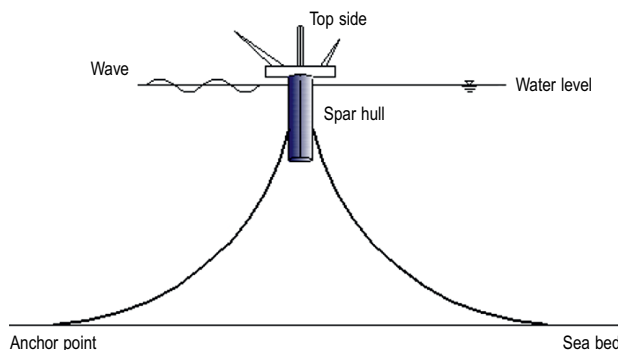


Fig. 1. Sketch of floating spar platform

timate the drag and inertia for all of the six degrees of freedom. The static coupled problem is solved by Newton's method. In order to incorporate high degrees of nonlinearities, an iterative time domain numerical integration is required to solve the equation of motion and to obtain the response time histories. The Newmark- $\beta$  time integration scheme with iterative convergence has been adopted for solving the coupled dynamic model. The equation of motion for the spar–mooring system having the equilibrium between inertia, damping, restoring and exciting forces can be accumulated as shown in Eq. (1).

$$[M]\{\ddot{X}\} + [C]\{\dot{X}\} + [K]\{X\} = \{F(t)\}, \quad (1)$$

where:  $\{X\}$  = 6 DOF structural displacement vector;

$\{\dot{X}\}$  = Structural velocity vector,

$\{\ddot{X}\}$  = Structural acceleration vector;

$[M]$  = Total mass matrix =  $[M]^{Spar+Mooring} + [M]^{Added\ mass}$ ;

$[C]$  = Damping Matrix =  $[C]^{Structural\ damping} + [C]^{Hydrodynamic\ damping}$ ;

$[K]$  = Stiffness matrix =  $[K]^{Elastic} + [K]^{Geometric}$

The six degrees of freedom (DOF) structural displacements are represented by  $\{X\}$  and the dot symbolizes differentiation with respect to time. The total spar–mooring mass matrix of the system consists of structural mass and added mass components. The structural mass of the spar–mooring system is made up of elemental consistent mass matrices of the moorings and lumped mass properties of the rigid spar hull. The lumped mass properties are assumed to be concentrated at the CG of a spar hull. The added mass of the structure occurs due to the water surrounding the entire structure. Considering the oscillation of the free surface, this effect of variable submergence is simulated as per Wheeler's approach. The total stiffness matrix element  $[K]$  consists of two parts, the elastic stiffness matrix  $[K_E]$  and the geometrical stiffness matrix  $[K_G]$ . The overall damping to the system is contributed by structural and hydrodynamic damping. The major damping is induced due to the hydrodynamic effects. It may be obtained if the structure velocity term in the Morison equation is transferred from the force vector on right hand side to the damping term on the left hand side in the governing equation of motion. The structural damping follows Eq. (2), in which  $\xi$  signifies structural damping ratio,  $\Phi$  is modal matrix,  $\omega_i$  denotes natural frequency and  $m_i$  implies the generalized mass.

$$\Phi^T [C]^{Structural} \Phi = [2\xi\omega_i m_i]. \quad (2)$$

### 1.1. Idealization of mooring line

The configuration of a mooring line is described by a 3-D Cartesian coordinate system in terms of a vector,  $\vec{X}(s,t)$ , which is a function of  $s$ , the deformed arc length along the mooring line (Fig. 2a).  $t$ ,  $n$ ,  $b$  are unit vectors in mooring tangential, normal and bi-normal direction respectively at Cartesian coordinate system. The internal

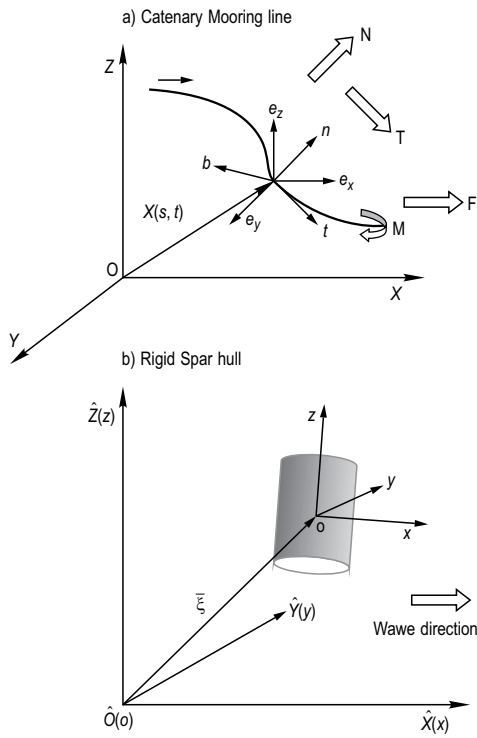


Fig. 2. Coordinate System of mooring line and spar hull

state of stress at a mooring point is described fully by the resultant force and the resultant moment acting at the centreline of the mooring line. The external forces applied on a catenary mooring line involve the gravity forces, hydrostatic forces and hydrodynamic forces. The wave force  $F_m(X, Z, t)$  per unit length of mooring line acting on a single mooring line can be derived as Eq. (3). Terminologies not mentioned here have been introduced in notations.

$$F_m(X, Z, t) = F_{Gravity} + F_{Inertia} + F_{Drag} + F^{F-K}_{Sea\ water} \quad (3)$$

The aforementioned force can be calculated by:

$$F_m = (\rho_w A_m - \rho_i A_i - \rho_t A_t) g e_z + \rho_w A_m (I + C_{Mn} N + C_{Mt} T) (\ddot{u} - \ddot{X}) + \frac{1}{2} \rho_w D C_{Dn} N (\dot{u} - \dot{X}) |N(\dot{u} - \dot{X})| + \frac{1}{2} \rho_w D C_{Dt} T (\dot{u} - \dot{X}) |T(\dot{u} - \dot{X})|. \quad (4)$$

The velocity  $\dot{u}$  and acceleration  $\ddot{u}$  in Eq. (4) are calculated from an appropriate wave theory. Prime indicates that the derivative is being done with respect to the arc length  $s$  of a mooring line. The subscripts  $w$ ,  $i$  and  $t$  denote the sea water, the fluid inside the mooring line and the mooring line tube itself. The term,  $I$  is identity matrix. Transfer matrices of tangential forces,  $T$  is defined by  $T = X'^T X'$  and transfer matrices of normal forces,  $N = I - T$ . As the motion of the structure is considered, there will be addition of some force exerted per unit length acting due to structural acceleration of mooring line element, which is equivalent to  $\rho_w A_m \ddot{X}$ . Adding this term

in Eq. (4), the total force acting on a mooring line is given by Eq. (5).

$$F_m = (\rho_w A_m - \rho_i A_i - \rho_t A_t) g e_z + \rho_w A_m (I + C_{Mn} N + C_{Mt} T) \ddot{u} - \rho_w (C_m - 1) A_m \ddot{X} + \frac{1}{2} \rho_w D C_{Dn} N (\dot{u} - \dot{X}) |N(\dot{u} - \dot{X})| + \frac{1}{2} \rho_w D C_{Dt} T (\dot{u} - \dot{X}) |T(\dot{u} - \dot{X})|. \quad (5)$$

The virtual mass matrix is simplified as:

$$[M]^{Mooring} = (\rho_t A_t + \rho_i A_i) I + \rho_w A_m C_{Mn} N + \rho_w A_m C_{Mt} T. \quad (6)$$

Hence, the dynamic equilibrium equation of a mooring line can be obtained as Eq. (7).

$$[M]^{Mooring} \{ \ddot{X} \} + \left( \frac{2 \xi \omega_{im} m_{im}}{\Phi T \Phi} + [C]^{hydrodynamic} \right) \{ \dot{X} \} + ([K]_E^{Mooring} + [K]_G^{Mooring}) \{ X \} = (\rho_w A_m - \rho_i A_i - \rho_t A_t) g e_z + \rho_w A_m (I + C_{Mn} N + C_{Mt} T) \ddot{u} - \rho_w (C_m - 1) A_m \ddot{X} + \frac{1}{2} \rho_w D_m C_{Dn} N (\dot{u} - \dot{X}) |N(\dot{u} - \dot{X})| + \frac{1}{2} \rho_w D_m C_{Dt} T (\dot{u} - \dot{X}) |T(\dot{u} - \dot{X})|. \quad (7)$$

## 1.2. Idealization of spar hull

In the derivation of motion equations of a floating rigid body two coordinate systems have been implemented. Coordinate system  $\hat{o}\hat{x}\hat{y}\hat{z}$  is a space-fixed coordinate system, while  $oxyz$  is the body-fixed coordinate system moving with the body. The origin  $o$  can be the centre of gravity ( $g$ ) or any point fixed on the body. The body-fixed coordinate  $oxyz$  coincides with  $\hat{o}\hat{x}\hat{y}\hat{z}$  when the body is at its initial position (Fig. 2b). The third coordinate system  $OXYZ$  which is a space-fixed coordinate system with  $OXY$  plan lying on the free surface and  $Z$ -axis positive upward is also introduced as a reference coordinate system. Incoming waves are applied in this space-fixed reference coordinate system. Therefore, the total force  $F_s(X, Z, t)$  per unit length on spar hull cylinder can be derived as Eq. (8).

$$F_s(X, Z, t) = F_{Gravity} + F_{nInertia} + F_{nDrag} + F_{Axial}, \quad (8)$$

where:

$$F_s = -\rho_t g A_s e_z + \rho_w A_s C_M (\ddot{u}_n - \ddot{X}_n) + \rho_w A_s C_M \ddot{u}_n + \frac{1}{2} \rho_w D_s C_D (\dot{u}_n - \dot{X}_n) |(\dot{u}_n - \dot{X}_n)| + \rho_w \iint_{S_B} \left( \frac{\partial(\varphi^{(1)} + \varphi^{(2)})}{\partial t} + \frac{1}{2} |\nabla \varphi^{(1)}|^2 \right) ds + C_{mt} \rho_w \frac{4}{3} \left( \frac{D_s}{2} \right)^3 [\ddot{u}_t - \ddot{X}_t] + \frac{1}{2} \rho_w C_{Dt} A_s (\dot{u}_t - \dot{X}_t) |(\dot{u}_t - \dot{X}_t)|. \quad (9)$$

Inclusion of the added forces considering motion of structure modifies the total force acting on the spar hull as Eq. (10).

$$\begin{aligned}
F_s = & -\rho_t g A_s e_z + \rho_w A_s C_M (\ddot{u}_n - \ddot{X}_n) + \\
& \rho_w A_s C_M \ddot{u}_n + \frac{1}{2} \rho_w D_s C_D (\dot{u}_n - \dot{X}_n) \left| (\dot{u}_n - \dot{X}_n) \right| + \\
& \rho_w \iint_{S_B} \left( \frac{\partial(\varphi^{(1)} + \varphi^{(2)})}{\partial t} + \frac{1}{2} \left| \nabla \varphi^{(1)} \right|^2 \right) ds + \\
& C_{mt} \rho_w \frac{4}{3} \left( \frac{D_s}{2} \right)^3 \left[ \ddot{u}_t - \ddot{X}_t \right] + \\
& \frac{1}{2} \rho_w C_{Dt} A_s (\dot{u}_t - \dot{X}_t) \left[ (\dot{u}_t - \dot{X}_t) \right] + \rho_w A_s \ddot{X}_n. \quad (10)
\end{aligned}$$

Therefore, the equation of motion for the spar hull leads to

$$\begin{aligned}
[M_s + M_a s] \{ \ddot{X} \} + \left( \frac{2\xi \omega_{is} m_{is}}{\Phi^T \Phi} + [C]^{hydrodynamic} \right) \{ \dot{X} \} + \\
([K]_E^{Spar} + [K]_G^{Spar}) \{ X \} = -\rho_t g A_s e_z + \\
\rho_w A_s C_M (\ddot{u}_n - \ddot{X}_n) + \rho_w A_s C_M \ddot{u}_n + \\
\frac{1}{2} \rho_w D_s C_D (\dot{u}_n - \dot{X}_n) \left| (\dot{u}_n - \dot{X}_n) \right| + \\
\rho_w \iint_{S_B} \left( \frac{\partial(\varphi^{(1)} + \varphi^{(2)})}{\partial t} + \frac{1}{2} \left| \nabla \varphi^{(1)} \right|^2 \right) ds + \\
C_{mt} \rho_w \frac{4}{3} \left( \frac{D_s}{2} \right)^3 \left[ \ddot{u}_t - \ddot{X}_t \right] + \\
\frac{1}{2} \rho_w C_{Dt} A_s (\dot{u}_t - \dot{X}_t) \left[ (\dot{u}_t - \dot{X}_t) \right] + \rho_w A_s \ddot{X}_n. \quad (11)
\end{aligned}$$

### 1.3. Equation of motion for a spar–mooring system

Formation of equation of motion for a spar platform, which combines a spar hull and mooring lines in a single integrated system can be expressed in Eq. (12).

$$\begin{aligned}
[M + M_a]^{Spar+Mooring} \{ \ddot{X} \} + \left( \frac{2\xi \omega_i m_i}{\Phi^T \Phi} + \right. \\
[C]^{hydrodynamic} \left. \right) \{ \dot{X} \} + ([K]_E^{Spar+Mooring} + \\
[K]_G^{Spar+Mooring}) \{ X \} = (\rho_w A_m - \rho_i A_i - \rho_t A_t) g e_z + \\
\rho_w A_m (I + C_{Mn} N + C_{Mt} T) \ddot{u} - \rho_w (C_m - 1) A_m \ddot{X} + \\
\frac{1}{2} \rho_w D_m C_{Dn} N (\dot{u} - \dot{X}) \left| N (\dot{u} - \dot{X}) \right| + \\
\frac{1}{2} \rho_w D_m C_{Dt} T (\dot{u} - \dot{X}) \left| T (\dot{u} - \dot{X}) \right| - \rho_t g A_s e_z + \\
\rho_w A_s (\ddot{X}_n + C_M (2\ddot{u}_n - \ddot{X}_n)) + \\
\frac{1}{2} \rho_w D_s C_D (\dot{u}_n - \dot{X}_n) \left| (\dot{u}_n - \dot{X}_n) \right| + \\
\rho_w \iint_{S_B} \left( \frac{\partial(\varphi^{(1)} + \varphi^{(2)})}{\partial t} + \frac{1}{2} \left| \nabla \varphi^{(1)} \right|^2 \right) ds + \\
C_{mt} \rho_w \frac{4}{3} \left( \frac{D_s}{2} \right)^3 \left[ \ddot{u}_t - \ddot{X}_t \right] + \\
\frac{1}{2} \rho_w C_{Dt} A_s (\dot{u}_t - \dot{X}_t) \left[ (\dot{u}_t - \dot{X}_t) \right]. \quad (12)
\end{aligned}$$

A rigid beam element is considered to model the cylindrical (Rasiulis, Gurkšnyš 2010) spar hull connecting

its centre of gravity, riser reaction points and mooring lines fair leads. The radii of gyration and the cylinder mass are defined at C.G. The spar platform is associated to the elastic mooring lines by means of six springs (three for translation and three for rotation). The stiffness of translation springs is very high; whereas the stiffness of rotational springs is very low simulating a hinge connection. This model handles all nonlinearities, loading and boundary conditions. The effect of riser in coupling has been ignored. The equation of motion has been solved using the commercial finite element code ABAQUS (2006). It has the capability of modelling slender and rigid bodies with realistic boundary conditions, including fluid inertia and viscous drag (Islam 2013). The mooring lines are modelled as three dimensional tensioned hybrid beam elements. It includes the nonlinearities due to low strain large deformation and fluctuating pretension. It is hybrid because it employs the mixed formulation involving six displacements and axial tension as nodal degrees of freedom. The axial tension maintains the catenary shape of the mooring line. Beam elements experience the wave forces due to Morison's equation. The self-weight and axial tensions are duly incorporated.

Three dimensional stiffness matrixes in ABAQUS are capable of including geometric stiffness matrix with elastic stiffness matrix.  $[K_G]$  models the large deformation associated with the mooring configuration. The ABAQUS/AQUA module appropriately models an off-shore environment. It is capable of simulating the hydrodynamic loading due to a wave. An automatic time interval ( $\Delta t$ ) incrementation solution scheme representing Newmark- $\beta$  approach is selected. The scheme uses half-step residual control to ensure an accurate dynamic solution. The half-step residual means the equilibrium residual error (out-of-balance forces) halfway through a time increment. For a continuum solution, the equilibrium residual should be moderately small related to significant forces in the problem. This half-step residual check is the basis of the adaptive time interval incrementation scheme. If the half-step residual is small, the accuracy of the solution is high and the time step can be increased safely; conversely, if the half-step residual is large, the time step taken in the solution ought to be reduced.

## 2. Numerical study and discussion of results

A modelled spar platform has been chosen allowing coupling of a spar–mooring system subjected to ocean waves in 1018 m deep water. Sea-state having “ $W_H$ ” (wave height) and “ $W_p$ ” (wave period) of 17.15 m and 13.26 s has been considered. The mechanical and geotechnical properties of the spar–mooring system under study are given in Table 1.

Table 2 shows the hydrodynamic characteristics of the marine environment. Mooring tensions are assumed to be equally distributed in all the four mooring lines. The spar hull is expected to behave like a rigid body. When the wave forces act on the entire structure, partic-

Table 1. Mechanical and Geometrical Properties of Spar-mooring system

Element	Properties	
Spar (Classic JIP Spar)	Length	213.044 m
	Diameter	40.54 m
	Draft	198.12 m
	Mass	2.515E8 kg
	Mooring Point	106.62 m
	No. of Nodes	17
	No. of Elements	16
	Type of Element	Rigid beam element
Sea water	Depth	1018m
	Density	1026 kg/m <sup>3</sup>
Mooring line	No. of Moorings	4
	Stiffness (EA)	1.501E+09 N
	Length	2000 m
	Mass	1100 kg/m
Mooring line	Mooring line pre-tension	1.625E+07 N
	No. of Nodes	101
	Element Type	Hybrid beam element

Table 2. Hydrodynamic properties

Structural element	Hydrodynamic coefficient	
Spar	Drag coefficient	0.6
	Inertia coefficient	2.0
	Added mass coefficient	1.0
	Drag coefficient in vertical direction	3.0
Mooring line	Drag coefficient	1.0
	Inertia coefficient	2.2
	Added mass coefficient	1.2

ipation of mooring lines in the overall response is well depicted. The variable boundary conditions due to mooring anchor point are appropriately incorporated. Due to the ideal modelling, the solution is having difficulty in convergence. Responses of spar and mooring lines under extreme regular wave have been evaluated.

The coupled form of structural modelling predicts true behaviour of spar–mooring system. This approach yields dynamic equilibrium between the forces acting on the spar and the mooring line at every time station. The computational efforts required for the coupled analysis including all mooring lines are substantial. The ability for more accurate prediction of platform motions by coupled analysis approach may consequently contribute to a smaller and comparatively less expensive spar–mooring system and hence a lighter spar platform through a lessening in payload requirements. The excursion time histories are found for sufficient length of time so that the response attains their steady state. To understand the mooring damping and coupling effect, long range responses are obtained. The responses in terms of surge, heave, pitch and mooring line tension are plotted for 17.15 m 13.26 s wave loading. The sea state has been

Table 3. Severe dynamic stresses (Jameel, Ahmad 2011)

Sea State	$W_H$ (m)	$W_P$ (sec)	RMS stress (MPa)	Probability of occurrence
S1	17.15	13.26	123.81	0.0000003
S2	15.65	12.66	122.74	0.0000023
S3	14.15	12.04	122.34	0.0000143
S4	12.65	11.39	121.88	0.0000798
S5	11.15	10.69	121.38	0.0004057
S6	9.65	9.94	120.09	0.0018712
S7	8.15	9.14	119.67	0.0077382
S8	6.65	8.26	118.98	0.0282212
S9	5.15	7.26	117.89	0.0885110
S10	3.65	6.12	117.46	0.2283116
S11	2.15	4.69	116.82	0.4354235
S12	0.65	2.58	115.93	0.2094203

defined as critical by Jameel and Ahmad (2011). Among all the sea states of their study (Table 3), sea state S1 (17.15 m wave height and 13.26 s wave period) is large in loading magnitude but lowest in probability of occurrence. Therefore, in this study, the sea state S1 has been selected for the assessment of the structural behaviour under extreme wave loading.

## 2.1. Validation of present coupled spar–mooring system model

The validation of the present model has been done with experimental study in OTRC wave basin, Texas A & M University conducted by Chen *et al.* (2001) with good agreement. Chen *et al.* (2001) have stated the difference of the maximum net tension in four mooring lines at the fair lead position, changing against various static off-sets in surge direction. The responses are obtained under regular wave loading of  $W_H = 6$  m;  $W_P$  and 14 s in 1018 m deep water condition. Fig. 3 shows the identical mooring line tension response variation with the Spar off-set at range 0–10m for the same deep water sea state. The illustration shows a bit difference with Chen *et al.* (2001) for all the off-sets ranging after 10 m to 25 m. However, the trend of the results is rather matching. The variation in the numerical values of net tension is mainly due to the basic difference in mathematical model. The present

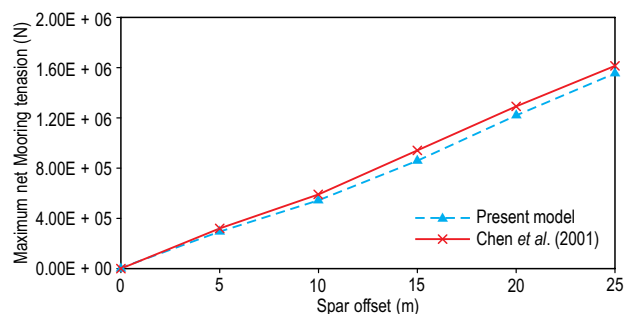


Fig. 3. Assessment of present model output with experimental result

study takes into account, the actual integrated coupling of entire structure by finite element assembly considering all major nonlinearities, while Chen *et al.* (2001) did it differently. The identical values attained in the present study confirm the validity of the fully coupled integrated model. Furthermore, it indicates that for the required state of equilibrium, the boundary conditions are appropriately implemented.

The comparison of natural time periods between Chen *et al.* (2001) and the present study has been carried out as well. Free vibration analysis of a spar platform is performed. Lanczos method has been used to obtain the natural frequencies and corresponding mode shapes. The natural periods obtained by Chen *et al.* (2001) are 331.86, 29.03 and 66.77 seconds in surge, heave and pitch respectively at 318 m water depth, which is very close with the present model result at the same depth with corresponding values of 323.97, 25.60 and 59.48 seconds. Furthermore, in 1018 m water depth, the natural periods obtained by the present simulation are 341.97, 22.60 and 43.48 in surge, heave in pitch direction. In comparison to the experimentally measured values (Chen *et al.* 2001), the present model values are expected. Due to the increase of water depth, surge time period increases but heave and pitch time periods decrease. These satisfactory assessments in consistent manner prove the accuracy of the present model.

## 2.2. Spar responses

### 2.2.1. Surge at platform level

The time series of surge response due to the sea state S1 at the deck level and CG level of the spar platform are shown in Figs 4 and 5, respectively. The peak of the surge response at deck level for the sea state S1 ranges from +22.50 m to −14.11 m. The nature of the surge at the deck level is predominantly periodic as shown in Fig. 4. Pitch motion (Fig. 6) occurs simultaneously with surge and attracts significant wave energy close to the pitch frequency. Surge response requires huge energy input because of large inertia and hence does not get excited. However, pitching motion occurring with surge gets excited easily. The surge response at the deck level is dominated by the pitching motion of the hull with insignificant excitation of surge mode. It is mainly due to the coupling of surge and pitch. Effect of non-linearity is not very strong on the surge response.

### 2.2.2. Surge at the spar centre of gravity

The translational response in surge direction at the centre of gravity of a spar is shown in Fig. 5 for 1018 m water depth. It shows a marked difference in surge behaviour in comparison to the same at platform level (Fig. 4). At CG level, it oscillates in similar regular pattern as the platform level responses. However, the fluctuations of the surge are small in value compared to the deck level excursions. There are continuous fluctuations of wave frequency, showing the pronounced non-linear behaviour. The mean value of surge shows a lateral shift of a spar by 4.25 m.

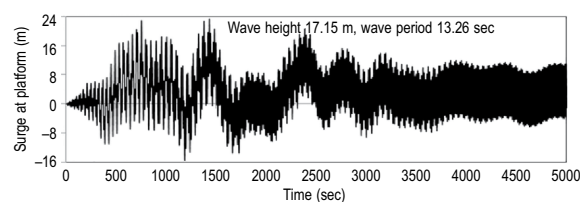


Fig. 4. Spar translational motion response in surge direction at deck level

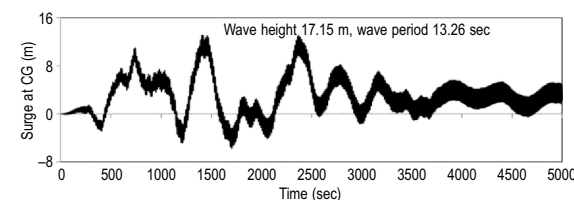


Fig. 5. Spar translational motion response in surge direction at CG

The surge response at deck level attains steady state at around 4000 sec. However, the response at CG level shows that the whole system needs more time to damp out the initial transient effect.

### 2.2.3. Spar response in heave direction

The heave response directly influences the mooring tensions and other operations. The heave response under regular wave for sea state S1 is shown in Fig. 6. The time history shows the cluster of reversals occurring at varying time intervals. The phenomenon displays regularity in the response behaviour. Larger magnitudes of heave responses occur earlier for the S1 wave loading. The maximum heave response of 1.6 m occurs around 1300 sec. The steady state is attained approximately at 3500 s of wave loading. The heave response fluctuates about the mean position oscillating from smaller to larger amplitudes and repeating the same trend onwards all through the time history for both cases. The fluctuations gradually increase from narrow to broad by 20%. Reaching the peak, it gradually reduces by 10% and again increases ensuring the similar trend.

Though the heave response attains steady state around 2400 sec, its value of oscillations gradually decreases till 4500 sec. This shows the damping of heave response due to longer and heavier mooring lines in deep water conditions. The relatively low value of heave for extreme wave loading also indicates the suitability of a spar platform for harsh deep water environment.

### 2.2.4. Spar response in pitch direction

The pitch behaviour of a spar hull subjected to regular sea waves is illustrated in Fig. 7. The time history of pitch response shows regular fluctuations initiating from zero up to peak of  $\pm 0.10$  rad. The pitch responses reduce periodically and again increase through taking energy. For the sea state S1, the steady state is observed within 1 hour. The significant value of pitch response leads to

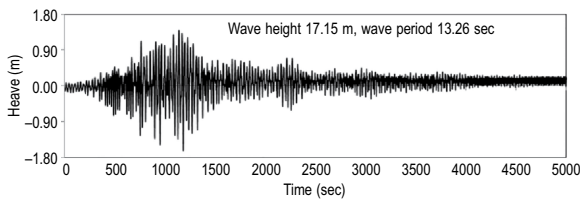


Fig. 6. Spar translational motion response in heave direction

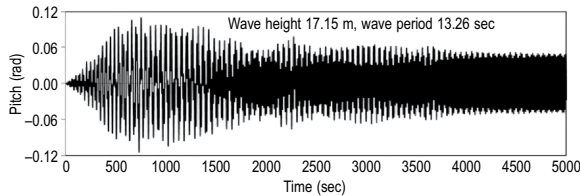


Fig. 7. Spar rotational motion response in pitch direction

a significant surge at the deck level. It is coupled with the surge of rigid hull, which otherwise is of small magnitude but gets enhanced due to the pitch input. This is why the surge time history shows the maximum peak at pitch frequency. The pitch time history also shows similar behaviour as surge time history.

#### 2.2.5. Maximum tension response in mooring lines

The response of mooring lines plays an important role in the coupled dynamic analysis of the spar platform. The regular wave loads simultaneously act on the hull and mooring lines. The analysis of this structure yields the coupled response in the true sense. Designed pretension in each mooring line of the present problem is  $1.625 \text{ E} + 07 \text{ N}$  (Table 1). A mooring line shows the regular behaviour of tension when subjected to the sea state S1 (Fig. 8). The surge response also causes increase in tension. The mooring line 1 is positioned in the direction of wave propagation before the spar hull. It is worth mentioning that the mooring line 1 experiences the maximum tension to support surge in the forward direction. Figs 8–9 show the tension fluctuations when the mooring line 1 and the mooring line 3 stretch respectively due to surge response. The tension fluctuation is of complex periodic nature showing minor ripples near the peaks. For both of these mooring lines at the regular wave periodic behaviour is governed.

The maximum tension time history in the mooring line 3 is shown in Fig. 9. As mentioned earlier, the mooring line 1 is in the direction of wave. It stretches due to wave action causing the mooring line 3 to slack. This phenomenon of stretch and slack repeats alternatively. Both of the mooring lines attain steady state approximately after 3500 sec, the fluctuations of pretension ranges from  $1.50 \text{ E} + 07 \text{ N}$  in the mooring line 3. The oscillation pattern for both mooring lines is identical. Compared to the mooring line 1, the mean value of tension fluctuations for the mooring line 3 is relatively less. This behaviour is expected because the mooring line 3 slacks due to extreme wave loading.

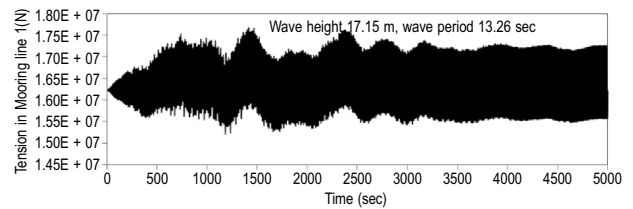


Fig. 8. Maximum tension in mooring line 1

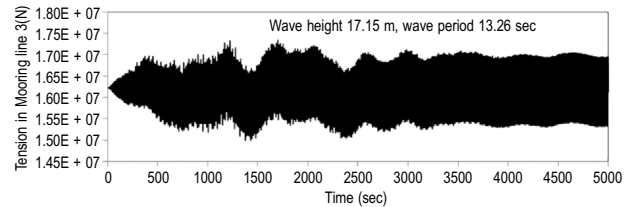


Fig. 9. Maximum tension in mooring line 3

## Summary and conclusions

For deep water offshore exploration, spar platforms have been recognized as efficient and economical structures. The finite element model for coupled analysis of spar and its mooring system developed in this study is capable of handling all nonlinearities, loading and boundary conditions. The conclusions of this study are as follows:

- 1) The spar response gets significantly modified and mean position of oscillations gets shifted for the extreme wave loading. For extreme regular wave, of  $W_H = 17.15 \text{ m}$ ,  $W_p = 13.26 \text{ sec}$  at 1018 m water depth, the surge, heave and pitch responses are predominantly excited.
- 2) The fluctuations of surge at the spar CG are small in value compare to the deck level excursions. The mean value of the surge at CG shows a lateral shift of the spar by 4.25 m.
- 3) The response time histories in surge, heave, pitch and the maximum mooring tension gradually decreases even after attaining the steady state. It is because of damping due to heavier and longer mooring lines in the coupled spar–mooring system under deep water conditions.
- 4) The relatively lesser values of response time histories in surge, heave, pitch and the maximum mooring tension under extreme wave loading shows the suitability of spar platform for deep water harsh and uncertain environmental conditions.

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## Notations

- $A_i$  = Inner cross-sectional area of the mooring line;  
 $A_m$  = Outer cross-sectional area of the mooring line;  
 $A_s$  = Cross-sectional area of the spar hull;  
 $A_t$  = Structural cross-sectional area of the mooring line;

$C_D$  = Drag coefficient;  
 $C_M$  = Inertia coefficient;  
 $C_{mt}$  = Added-mass coefficient of the Spar cylinder bottom;  
 $C_{Dt}$  = Drag coefficient of the Spar cylinder bottom;  
 $D_m$  = Diameter of mooring line;  
 $D_s$  = Diameter of the spar hull;  
 $e_x$  = Unit vector in X-axis;  
 $e_y$  = Unit vector in Y-axis;  
 $e_z$  = Unit vector in Z-axis;  
 $P_w$  = Pressure of the sea water;  
 $P_i$  = Pressure of the internal fluid;  
 $\varphi^{(1)}$  = First-order potential of incident waves;  
 $\varphi^{(2)}$  = Second-order potential of incident waves;  
 $\rho_w$  = Mass density of the sea water;  
 $\rho_m$  = Mass per unit mooring line;  
 $\rho_s$  = Mass density of the spar.

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